



**UNIVERSITY OF SWAZILAND  
MAIN EXAMINATION PAPER**

**PROGRAMMES:** BSc. ABE 3, and BSc LWM 3 OLD (T)

**COURSE CODE:** ABE 303

**TITLE OF PAPER:** FLUID AND SOIL MECHANICS

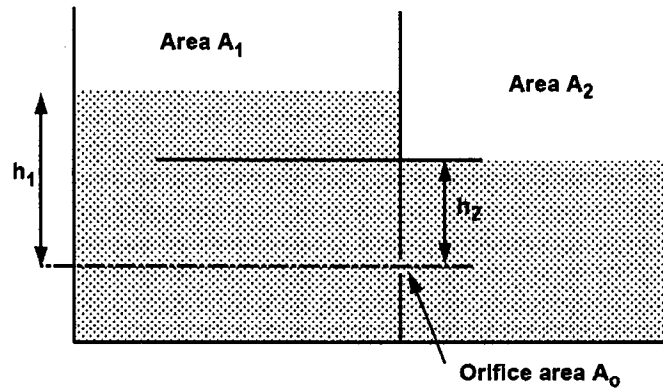
**TIME ALLOWED:** TWO (2) HOURS

**INSTRUCTIONS:** ANSWER QUESTION ONE AND ANY TWO OTHER  
QUESTIONS

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THE CHIEF INVIGILATOR**

**QUESTION 1**

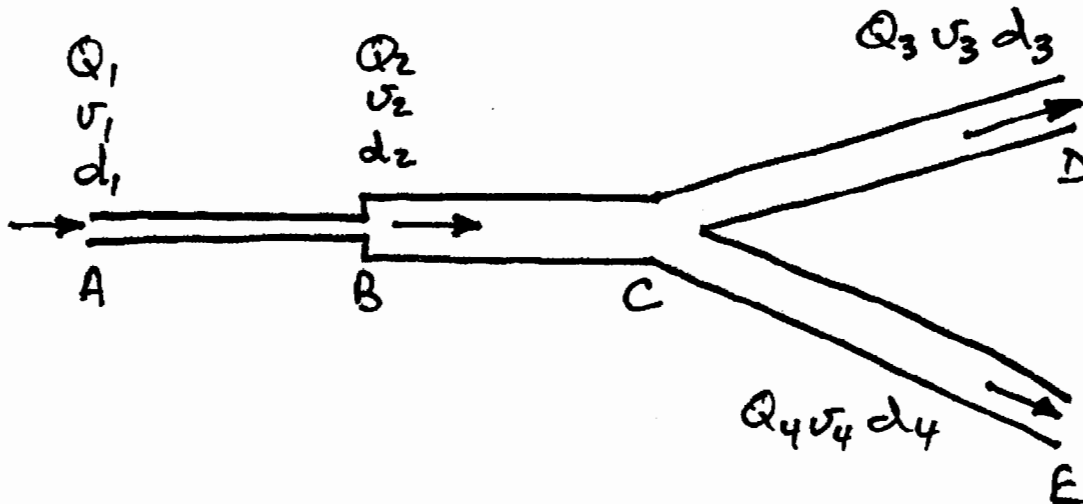
a. Two vertical cylindrical tanks of 5.0 m and 3.0 m diameter contain water. They are joined near their bases by a pipe of diameter 5cm which is short enough to be considered an orifice with  $C_d$  of 0.6. If the 3.0 m diameter tank initially has a level 2.0 m higher than the other, calculate how long it will take for the levels to become equal in each tank.



- i. Determine the *ideal velocity* at point 2 (at the centre of the orifice). [5 marks]
  - ii. Calculate the *actual discharge* at point 2 [5 marks]
  - iii. Calculate how long it will take for the levels to become equal in each tank [10 marks]
- b. Define effective stress and state its significance in practical soil mechanics problems. [10 marks]
- c. Outline the role of the Proctor test in civil engineering works such as embankments or earth dams. [10 marks]

**QUESTION 2**

- a. A certain town receives its water directly from a water tower. If the top of the water in the tower is 26.0 m above the water faucet in a house, what should be the water pressure at the faucet? (Neglect the effects of other users). [10 marks]
- b. Outline the Mohr- Coulomb failure criterion [10 marks]
- c. Water flows from point *A* to points *D* and *E* as shown in the figure below. Some of the flow parameters are known as shown in the Table. Determine the unknown parameters.



Section	Diameter (mm)	Flow rate (m <sup>3</sup> /s)	Velocity (m/s)
AB	300	?	?
BC	600	?	1.2
CD	?	$Q_3 = 2Q_4$	1.4
CE	150	$Q_4 = 0.5Q_3$	?

[10 marks]

**QUESTION 3**

a. Estimate the energy head lost along a short length pipe suddenly enlarging from a diameter of 350 mm to 700.0 mm which discharges 700.0 l/s of water. If the pressure at the entrance of the flow is 105.0 N/m<sup>2</sup>, find the pressure at the exit. [10 marks]

b. A concrete lined channel has base width of 5.0 m and the sides have slopes of 1:2. Manning's *n* is 0.015 and the beds slope is 1:1000:

i. Determine the discharge, mean velocity and the Reynolds number when the depth of flow is 2.0 m. [10 marks]

ii. Determine the depth of flow when the discharge is 30.0 m<sup>3</sup>/s. [10 marks]

**QUESTION 4**

a. A vane of 250.0 *mm* and diameter 100.0 *mm* is used to measure the shear strength of a saturated soil. If the torque required to fail the vane is 518.0 *Nm*, calculate the apparent shear strength of the soil. **[5 marks]**

A test on the same soil was carried out using a vane of 300.0 *mm* length and diameter 100.0 *mm* and the torque at failure was 612.0 *Nm*. Calculate the ratio of shear strength in the vertical plane to shear strength in the horizontal plane. **[15 marks]**

b. Discuss the use of pump performance (characteristics) curves, outlining clearly how such are used for pump selection. **[10 marks]**

## APPENDIX

$$h_L = K'(\text{Re}^{\epsilon-2}) \left( \frac{L}{d} \right) \left( \frac{v^2}{2g} \right) = f \left( \frac{L}{d} \right) \left( \frac{v^2}{2g} \right)$$

$$t = \frac{A_1 A_2}{(A_1 + A_2) C_d A_o \sqrt{2g}} \left[ \sqrt{h_{\text{initial}}} - \sqrt{h_{\text{final}}} \right]$$

$$Q_{\text{actual}} = C_d A_1 A_2 \sqrt{\frac{2g \left[ \frac{p_1 - p_2}{\rho g} + z_1 - z_2 \right]}{A_1^2 - A_2^2}}$$

$$u_1 = \sqrt{\frac{2gh(\rho_{\text{man}} - \rho)}{\rho}}$$

$$Q_{\text{actual}} = C_d \frac{8}{15} B \sqrt{2g} \tan\left(\frac{\theta}{2}\right) H^{5/2}$$

$$\text{Re} = \frac{vL\rho}{\mu}$$

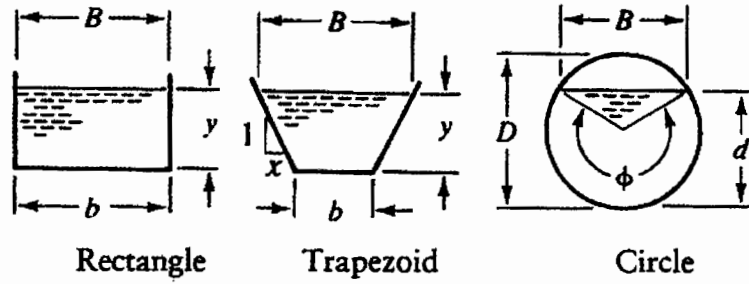
$$T = c_v (\pi d h) \frac{d}{2} + c_H \left( \pi \frac{d^2}{4} \right) \frac{2}{3}$$

$$h_f = \frac{32\mu Lv}{\gamma d^2}$$

$$\text{Re} = \frac{vR\rho}{\mu}$$

$$f = 64/\text{Re}$$

$$Q_{\text{actual}} = C_d \frac{2}{3} B \sqrt{2gH^{3/2}}$$



	Rectangle	Trapezoid	Circle
area, $A$	$by$	$(b + xy)y$	$\frac{1}{8}(\phi - \sin \phi)D^2$
wetted perimeter, $P$	$b + 2y$	$b + 2y\sqrt{1 + x^2}$	$\frac{1}{2}\phi D$
top width, $B$	$b$	$b + 2xy$	$\left(\sin \frac{\phi}{2}\right) D$
hydraulic radius, $R$	$\frac{by}{b + 2y}$	$\frac{(b + xy)y}{b + 2y\sqrt{1 + x^2}}$	$\frac{1}{4}\left(1 - \frac{\sin \phi}{\phi}\right) D$
hydraulic mean depth, $D_m$	$y$	$\frac{(b + xy)y}{b + 2xy}$	$\frac{1}{8}\left(\frac{\phi - \sin \phi}{\sin(1/2\phi)}\right) D$